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连续—非连续结构应力分析现状与进展

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摘 要: 对工程结构应力分析中采用的连续介质模型、非连续介质模型、连续——非连续介质力学模型作了系统介绍。三种模型物理数值模拟的各种理论及数值方法, 代表了当今结构应力分析的主流及发展方向。随着工程建设中高、大、精、新结构的出现, 表明这个古老而又崭新的命题仍有无限广泛的发展前景。从七个方面论述了它的现状及发展特征: 从过时观到现代新观念; 从连续到非连续; 从精确到数值近似; 从有限元到无单元; 从单一到统一; 从标量无网格到复变量无网格; 从独立到耦合, 全面介绍了其内容和方法。

关键词: 连续介质; 数值模拟; 应力分析; 力学模型

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STATE-OF-THE-ART OF STRESS ANALYSIS OF CONTINUOUS—DISCONTINUOUS STRUCTURES

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Abstract: Continuous, discontinuous, continuous—discontinuous medium mechanics models adopted in structural stress analysis are introduced systematically. Various theories and numerical methods in the three models which represent the mainstream and direction of stress analysis nowadays are discussed. With the advent of tall, large-scale, precise and new structures, research effort is still needed in the development of the three models. This paper describes the state of the art of stress analysis of continuous-discontinuous structures. It covers a great variety of issues including concepts, continuity to non-continuity, exact and approximate solutions and so forth.

Key words: continuous medium; numerical simulation; stress analysis; mechanics model

1 从过时到现代

工程结构应力分析科学领域内, 当前存在着巨大的挑战, 也面临着飞速发展的机遇。一方面随着现代工业技术的进步及人类挑战自然的努力, 在航空、航天、航海、能源、建筑、水利、交通、环境等技术领域出现了大量高、大、精、新结构, 如大型、高速飞机、轮船; 火箭壳体; 深埋地下工程;

高速铁路及机车; 三峡水电站; 青藏铁路冻土工程; 南水北调水电及岩石工程……等, 原有的理论、方法和手段已不能满足该类新型结构物对强度、刚度、稳定性及经济性、可靠性提出的更高技术要求。另一方面, 随着计算机技术的进步、推广及应用, 又为传统意义上的结构应力分析研究开辟了广泛的发展空间及应用领域。过去在学术界存在的一种

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结构应力分析相对于近代物理学成就而言毫无新意的过时观,已随着近代结构分析新理论体系的建立和计算机可视化技术、仿真模拟分析、多媒体网络技术的注入开创了生气勃勃的现代计算结构分析的新观念、新理论、新方法和新领域。

2 从连续到非连续

将研究对象用“连续介质”作为计算模型并用连续函数的解析方法研究其平衡及运动状态物理参数的数量关系(Ewler,1753),构成了物理学中连续介质力学的理论基础^[12,13]。由此而发展的液体力学、固体力学、气体力学等各个分支,皆以连续性假设为前提,对研究对象的物质组成(可统称为“材料”)及物理特性认识不断深化,各自形成了完整的理论体系,推动了工程技术不断进步。本文仅限于讨论可变形固体力学中的有关问题。

从一个螺丝钉到地球地壳,都是可变形的固体,工程科学中统称为“结构”。结构强度、刚度、稳定性是其研究内容的共性,按照对材料物理特性及结构性状的抽象,派生出了材料力学、结构力学、弹性力学、塑性力学、粘弹/塑性力学、流变体系力学、强度理论、流固耦合^[1,2,3,10,11].....等力学分支。将它用于金属、混凝土、岩土、合金、橡胶等各种材料,可建立两种力学模型:

无缺陷模型:连续介质宏观模型。自十七世纪中叶(R.Hooke 1666)至第二次世界大战(R.V.Mises, 1913 及 T.Von.Karman,1939),以弹性理论、塑性理论、稳定性理论为代表^[17,18,19,20];

有缺陷模型:非连续介质宏观模型。自二战至二十世纪 80 年代,以断裂力学(A.A.Griffith,1921)及损伤力学(L·M·Kachanov,1958)为代表^[32,35,36,37,38,39,44]。

以结构工程使用最广泛的混凝土材料强度问题为例,混凝土在承载前已存在大量的微裂缝、加渣、气泡、孔穴等,严格说来,不能认为是连续的整体。工作过程中,屈服、裂缝出现及扩展、丧失承载力、变形等物理现象都可用来定义“混凝土破坏”。将其作为连续宏观模型处理,实验表明:在多轴荷载作用下,混凝土的强度不仅仅与单独压、拉、剪应力有关。其强度准则必须考虑各个应力分量之间的相互关系对混凝土强度的影响,这种破坏准则应表达为应力状态的函数。由于混凝土材料的实际特性和强度是一个非常复杂的问题,它与骨料

和砂浆的物理力学性质、配合比、荷载特性等诸多因素有关,在不同的荷载条件下所表现出的承载力也不同,故迄今为止还没有哪一种数学模型可以描述真实混凝土在所有条件下的强度问题。即使构造了这样一种数学模型,也会由于其过于复杂繁琐而难于应用于工程实际中去^[61,62,63,241,242,247,248]。

2.1 强度准则

有关在多轴荷载作用下强度问题的研究^[59],最早始于伽得略于 17 世纪首先提出的最大主应力理论。但相当长时间混凝土的破坏理论发展较慢,只有在计算机推广应用后,在大量研究工作的基础上,混凝土的破坏理论才有了长足的发展^[263~268]。

按照其理论体系及发展年代,各种强度准则可归纳为表 1 所示^[234~236,253~255,257]。

西安交通大学俞宏茂教授于 1961 年提出了适用于等拉压强度材料的双剪应力屈服准则。历经三十多年的悉心研究,逐步发展为适用拉、压强度不同材料的双剪应力强度理论(1962, 1983~1985)、双剪帽子模型(1986),双剪多参数准则(1988~1990),最后形成了具有统一力学模型、统一理论及统一的数学表达式,并可适用于多种材料的统一强度理论(1991)。上世纪 80 年代后期,又将其推广应用于结构的弹塑性分析,发展形成了双剪弹塑性本构模型和统一弹塑模型、双剪统一粘弹塑性本构模型。近年来,又有一些研究人员在此基础上作了进一步的工作,如用于断裂力学和损伤力学中去^[258,259]。

双剪强度理论自成体系、内容丰富、描述统一、结构严谨,是我国学者对强度理论的重大贡献。它包含了国内外学者在过去的 20 多年所提出的一些主要强度理论,可用统一强度理论的形式概括描述。其理论及其应用价值不可低估,目前仍处于推广应用阶段。

2.2 本构关系

以连续介质宏观模型为基础的混凝土本构关系研究的理论体系包括:① 线弹性理论;② 非线性弹性理论;③ 弹塑性理论;④ 连续损伤力学理论;⑤ 断裂力学理论;⑥ 粘弹性理论;⑦ 内时理论^[260,261,262,263,266,267,268]。

本构关系是描述材料软科学性质的一般表达式,如破坏准则、应力应变关系、高温特性、腐蚀特性等等。本文所涉及到的本构关系,主要是在二、三轴复杂荷载下反映混凝土材料受力特性的应力应关关系。它是在某些力学理论基础上建立所描述

介质的数学表达式。研究途径大致分为两大类：其一是侧重于理论研究，即以连续介质力学为基础，建立可被实验验证的能表达真实材料基本特性的理想化模型；其二是侧重实验研究，即以实验数据为根据，通过拟合的方式，给出反映材料综合特性的经验表达式。由于混凝土材料特性的复杂性，在其本构关系的研究中，实验研究是第一位的。而根据混凝土材料的物理参数从纯理论的推导得出一个完整的本构关系是极其繁难的^[257,260,271,274]。

在线弹性理论基础上，通过描述材料随变形增长其弹性常数 E ， μ 发生变化从而反映应力—应变非线性特性，即为非线性弹性模型^[35,36,84,255,276]。由于

该模型形式简单、便于应用，可直接根据实验结果进行，故广泛应用于工程结构分析^[242,243,247,248]。比较有代表性的模型有：Darwin 和 Pecknold 模型、Kupfer 和模型、Ottonse 模型及在此基础上江见鲸等人提出的修正模型等等^[234,236]。另外，还有违背能量守恒定律适用于各向异性、正交各向异性、横向各向异性材料的超弹性本构模型、三次弹性本构模型、次弹性本构模型^[235]等等。

2.3 塑性条件 考虑材料塑性变形，必须确定以下基本原则，即：屈服条件、强化规律、流动法则、破坏准则^[266,245,230,234]。

表 1 混凝土强度准则理论体系及发展

Table 1 Theory system and development in strength criteria of concrete

理论构架	强度理论和准则	创建人	年代	适用范围、条件及特点
古典强度理论	最大主应力理论(第一强度理论)	Galelio	17 世纪	脆性材料
		Rankine	1876 年	拉伸断裂
	最大应变理论(第二强度理论)	E.Mariotto	1682 年	单轴拉、压
	Mohr-Coulomb 强度理论 (第三强度理论)	C.A.Coulomb	1713 年	混凝土受弯破坏
		O.Mohr	1900 年	金属延性材料及混凝土脆性材料
		Mohr-Coulomb	C. 20 初	混凝土、中等应力水平延性材料
		Druker-Prager	1952 年	克服了 M-C 准则破坏面不光滑缺点
	Tresca & Mises 屈服准则 (第四强度理论)	Tresca	1864 年	金属材料屈服，混凝土受压极限
		Von Mises	1913 年	材料进入塑性状态
三参数破坏准则	Bresle & pister 三参数破坏准则	B.Bresle K.S.Pister	1958	不能反映 I_3 的影响， $\tau_{oct} \sim \sigma_{oct}$ 在偏平面上是圆
	Willam Warnke 三参数破坏准则	K.J.Willam E.P.Warnke	1975	$\tau_{oct} \sim \sigma_{oct}$ 在偏平面上是椭圆，可退化为圆
	Menetrey-Willam 准则	Menetrey	1995	可退化为 V.Mises.D-P,M-C
四参数破坏准则	Reimann 强度准则	Reimann	1965	子午线为抛物线
	Ottosen 准则	N.S.Ottosen	1975	包含全部偏应力张量不变量，实验吻合良好
	Hsich-Ting-chen 破坏准则	S.S.Hsich E.C.Ting W.F.Chen	1979	偏平面上为非圆形，可退化为 D-P.Mises 及最大拉应力准则
五参数破坏准则	Willam-Warnke 五参数强度准则	K.J.Willam E.P.Warnke	1975	可描述低、高静水压力混凝土性能
	Kotsovos 强度准则	Kotsovos	1979	用指数型子午线和椭圆改进 W-W 准则的抛物线型子午线
	Podgorske 强度准则	Podgorski	1985	适用于混凝土、砂和粘土材料
	过一王五参数强度准则	过镇海 王传志	1991	适用于一切应力比和各种实验条件

(1) 屈服条件: 混凝土材料没有明显的屈服极限, 基于 Von Mises 屈服条件建立起来的相关联弹塑性模型称为 Prandtl-Reuss 材料; Drucker-Prager 考虑静水压力的影响对其作了改进, 建立了由第一、第二应力不变量 I_1, J_2 表示的屈服条件; Lary 将 Mohr-Colomb 条件与最大拉应力准则联合使用, 建立了 Lary 拉断 Mohr-Coulomb 材料的理想弹塑性模型。

(2) 强化规律。表示材料从屈服到破坏过程中变形发展的基本规律, 是塑性理论的核心。许多模型都是通过正确地描述强化规律得到相应的本构关系。1975 年 W.F.Chen 最早建立的混凝土 Chen-Chen 塑性本构模型, 把强化规律看作为破坏曲面的比例缩小, 韩大健经改进提出了非均匀强化模型。在此基础上, Murray 等提出了三参数硬化模型(1979); Hsieh-Chen 硬化模型; 俞茂宏(1992)双剪弹塑性硬化本构模型, 都进一步改进和完善了混凝土强化规律。

(3) 流动法则。反映了塑性应变增量与塑性的正交性。当塑性面取为强化面时为关联流动法则, 否则便是非关联流动法则。文献[269]通过分析认为关联流动法则不能正确预报混凝土塑性体积变化规律, 因而发展了非关联流动的本构模型。这一方面模型有: Vermeer-Borst(1984)非关联流动的本构模型、Vermeer-Bost(1984)非相关流理想弹塑性本构模型、Pietruszczak-Jiang-Mirza(1988)混合硬化模型、Han-Chen 混合硬化模型等。

(4) 内时理论。Valauris 于 1971 年提出的内时理论模型, 不以屈服面、硬化规律、流动法则为基本条件, 认为塑性、粘塑性等类耗散材料内任一点现时应力状态是该点邻域内整个变形和温度历史的泛函, 该历史用取决于变形中的材料特性和变形程度的内蕴时间标准 Z 来度量。其理论基础是以内变量理论为基本网络的不可逆热力学, 形式上代表着一种特殊形式的粘塑性理论。Bazant 等曾将其用到混凝土本构关系中, 概括了应变软化、非弹性体积膨胀、静止压力敏感性等基本特性, 但难以推广应用。

2.4 非连续损伤—断裂宏观模型

混凝土在自然状态下就是一种有缺陷的多孔隙介质, 在受荷载后, 其破坏是由微观裂缝的起裂—发展—扩展直至出现宏观裂缝的脆性断裂过程[38,39,40,41,45,47,61,62,63,68,69,70,71]。应用上述连续介质力学

理论建立的本构模型, 有广泛的使用价值, 但严格的说, 都不能反映混凝土材料的破坏本质。建立在非连续宏观模型基础上的损伤力学及断裂力学, 在内变量耗散理论及热力学基本定律基础上, 用微观、宏观、宏微观结合的方法的变革^[79,80,83]。近三十年来, 随着实验手段的改进及仿真模拟的发展, 又取得了长足进步, 各种理论方法著作, 揭示有缺陷损伤—破坏的内在规律, 从而为破坏准则及本构关系的研究开创了新的局面, 带来了认识其破坏机理、建立物理及数学模型、完善数值论文及研究成果不胜数^[40,45,48,49,50,56,57,58], 但能全面反映混凝土这种复合材料特征的成果, 尚不成熟, 本文不再引申和讨论。

综上所述, 目前还没有哪种理论被公认是完全符合混凝土材料特征的准确描述, 但各种理论又各具特色, 分别适用于一定条件及范围, 并取得了一些有工程价值的成果。在理论发展的过程中, 又出现了许多组合理论, 如弹塑性损伤断裂模型; 塑型断裂模型; 塑型损伤模型, 以及边界面模型、微平面模型、神经网络材料模型等。其结果是理想模型日趋复杂, 而对提高精度和在工程中推广应用方面贡献不大。

3 从精确到近似

工程结构的应力分析, 不管把实际问题理想化为什么样的物理模型, 在数学上, 都归结为在给定边界条件下的工程科学问题控制微分方程, 即微分方程的边值问题。统一数学描述为: $L\phi = q$ (在 D 域内); $\phi_s = \tilde{\phi}$ (在 S 边界上, $S \in D$); q 为已知或未知的 ϕ 的函数, L 为线性或非线性微分算子。

边值问题的解答, 可有三种途径: 解析解、数值解及实验解。解析解即边值问题的完全精确解, 对于弹性、塑性、粘性静动力学问题仅可对简单边界典型问题得到数学上的精确解答^[1,2,3,4]; 在计算机面世以前, 对复杂边界实际问题, 只能借助于机械、光学、电磁学等物理手段模拟其类比实验解答, 如电比拟法、光测法、薄膜比拟法等^[4,10,11]。连续介质问题的数值解答分为微分方程数值解及离散模型数值解两大类。微分方程数值解是将微分方程在数学上进到“离散”, 将其方程转化为线性或非线性代数方程, 从而把无限自由度(连续介质)用有限自由度(离散模型)表达, 求出满足边界条件的离散模型。在给定点(或网格)上近似值, 如数值积分法、

加权余量法、有限差分法、瑞利—李兹法、变分法、伽辽金法等；或将微分方程降阶为边界积分方程，用直接积分或间接积分的方法求边界积分方程的数值解。离散模型数值解，则是对计算模型(求解域)进行物理—几何上的分割，将连续体用有限元单元(域内或边界上)在有限个结点上联结为离散集合体，从而用代数方程求出结点或单元内的数值近似解^[5,6,7,8,9]。这种从精确解到近似解的求解过程，促进了工程技术的发展，是人类认识客观世界本质的一个飞跃。从客观意义上讲，它是定量评价未来客观世界一切过程的基本方法之一，具有不可估量的技术经济和社会发展的意义。

4 从单一到统一

不连续介质离散模型数值近似解最早出现于岩体工程中分片刚体模型，如刚体元法(Cundall,1971)及块体元法(石根华,1983)，不考虑单元变形，皆属于刚体离散单元模型。其实质都基于分片刚体模型，可反映岩体中的断层、夹层、裂隙、节理不连续结构面上的位移及运动性态^[21,22,24,25]。计入单元变形的变形体离散单元模型，如刚体弹簧元法(Kawai,1977)及变形块体元法(石根华,1992)皆是分片变形体模型^[26,27,28]。刚体弹簧元法是将块体元变形用累积在弹簧上的变形表示，其支配方程类似于有限元，故又称为刚性有限元法。变形块体元法采用不连续变形块体模型兼有有限元的特点，块体运动分析类似刚体离散单元，而每一时段内的求解类似于有限元。在上述思路基础上，卓家寿基于Kawai刚体弹簧元思路提出了界面元离散模型(1993)，以界面元替代弹簧反映不同类型单元的不连续变形并导出了各向异性体单元交界面上任一点的界面应力公式，可用来解决各向同/异性体连续—非连续介质工程结构的数值分析问题^[28,29,30,85,86,87]。而流形元法(石根华,1991；林德璋,1992)、无单元法或称无网格法(Belytschko,1994)，统一数值流形元法(石根华,1996)，则为求解统一连续—非连续问题开辟另一新的途径^[91,96,98,135]。

5 从有限元到无单元

自1960年R.W.Clough奠定了有限元法基本框架后，经过O.C.Zienkiewicz(1965)、E.L.Wilson(1973)等人在一般场问题及软件开发方面的卓越工

作^[5,6,7,8,9]，至1975年，K.H.Huebner在其“The Finite Element for Engineers”一书中说：“从为一种分析技术来讲，有限元法已达到不可能期望再有别的戏剧性的发展或突破了。当然未来的发展将包括在许多实际问题中更广泛的应用，增加对特殊重要方面的理解，进而提高和完善有限元法的基本技巧”^[5]。近三十多年来，J.H.Argyris、C.A.Brebbia、卡卞学璜等学者在数值物理模型、数学理论分析、各个领域中的推广应用及软件市场开发等方面的卓越贡献皆验证了Huebner的这一论断^[14,15,16]。

在细、微观上，所有变形固体都是非连续介质，宏观连续介质的非连续性，包含两类基本问题：其一是含有节理、裂缝构造的非连续体，破坏表现为不连续面张开及剪切滑移；其二是连续体中裂缝的产生和扩展。

随着耗散、分叉、分形、浑沌、模糊等理论的形成及发展，在连续介质有限元基本思想基础上，如上所述，自上世纪90年代初期，发展了非连续介质固体力学各种数值算法，进而发展了统一连续和非连续介质问题的数值方法^[30,27,91]。解决宏观非连续性问题，当前主要有五种技术手段，即：弹塑性断裂力学模型数值方法(FEM、BEM、WRM、COV等)；弹塑性损伤力学模型数值方法(PDM、BEM、WRM、COV等)；不连续块体模型数值方法(DEM、SEM、RSM、DDM、IEM)等；数值流形元法(FEM+COVER)；无单元或无网络法(NEM)。其中，数值流形元法是有限元的延伸、扩大发展^[96,97]。它引入了数学及物理两种网格，来定义数值解精度(如用三角形单元)及求解域边界及裂缝面；用数学及物理两种覆盖(Cover)来代替“单元”及“结点”；以三个物理覆盖之交集定义了“流形单元”，即用物理覆盖代替单元的节点，用覆盖的交线代替单元的边界，用覆盖的交集代替单元。被节理线、裂缝面、边界等隔离开的流形单元的公共线间可有不同节点，故可反映开裂，表现出非连续性^[279,280,281,282,203]。从而实现了连续性与非连续性的统一。而无单元法，则完全没有“单元”及网格的概念。

6 从无网格法到复变量无网格法

无网格法产生至今已逾二十余年，由于无网格法在场变量的逼近、边界条件的引入、能量泛出积分等方面存在的灵活性，相继出现了不同命名的多种无网格方法。在其发展过程中，多数学者所提出

的数学格式都是以移动最小二乘法形成无网格方法的逼近函数。表 2 对已见诸报导的各种方法及特点作了综合比较^[103~130,133~140]。

目前较流行的是无网格 Galerkin 法 (即 EFGM), 它以移动最小二乘法构造插值函数^[196,197], 从微分方程的弱变分原理出发, 导出求解问题的代数方程。其优点是自适应分析中不需重构网格; 前处理简单; 计算结果光滑连续; 数值精度较高。作为一种新兴的数值计算方法, 理论上具有开创性并初步形成了完整的构架及体系。但还存在着以下问题^[187,188,278]:

(1) 作为无需背景网格的点积分法还缺乏必要的理论支持, 精度有所降低;

(2) 运算中每一点都需计算一次形函数及其导数, 每次都涉及矩阵求逆及连乘, 在每个背景积分网格中皆需使用高阶高斯积分, 计算量大、费机时;

(3) 影响求解精度的因素多, 包括节点分布密度、基底函数的阶次、权函数的选取及其影响域大小; 边界条件引入的不确定等;

针对目前使用标量的移动最小二乘法形成的无网格方法配点过多的问题, 近期提出了对向量函数逼近的复变量移动最小二乘法^[131,277], 其优点是所取试函数中所含待定常数减少, 形成的二维问题的无网格方法可取较少的节点, 对任一场点而言, 其紧支域中所含的最小节点数就大大减少了; 同时不形成病态方程组、精度高, 所形成的无网格方法计算量小。

采用基于正交基函数的复变量移动最小二乘法建立的复变量无网格法, 将具有配点少、精度高、计算速度快的优点, 可有效地解决使用变量的无网格法配点过多。计算速度慢、容易形成病态方程组的缺点。为了较好地模拟裂纹尖端场的局部化现象, 在复变量无网格法基础上, 利用裂纹尖端位移场的解析解扩展复变量无网格法的基函数, 提出了扩展的复变量无网格方法, 可较好地解决含裂纹的问题, 提高了复变量无网格法求解裂纹问题的精度^[277]。

7 从独立到耦合

无网格法具有有限元法的一些优势, 但它的缺点正好是有限元法的优点, 采用二者耦合的方法, “取长补短、优势互补”, 将为其应用开拓广阔前景。

有限元法从理论——方法——应用——软件开发诸方面已相当完善, 随着 CAD 及可视化、多媒体、网络技术的发展及在计算分析中的应用, 有限元法可视化分析软件系统在数量上、应用价值上皆居主流和领导地位。国际知名的有限元软件约有 400 多种, 其中较著名的有十几个, 如 NASTRAN、MARC、ASKA、ADINA、GTSTRUDL、ANSYS、ABAQUS、SAP……等, 其强大的计算、设计内核及可视化程度相当高的前、后处理系统, 已被国际工程项目开发认可并作为标准^[283]。充分利用这一市场优势, 不断维护、完善、提高、扩大有限元软件的功能及使用范围, 将具有重大的技术经济价值。

把有限元法与加权残余法耦合或与边界元法耦合, 已取得了可喜成果^[209,225,284,285,286,287,288,290]。

Belytschko 最早提出了无网格法与有限元法的耦合方法, 将求解域划分为无网格子域及有限元法子域, 在两子域的界面元定义适当的耦合位移函数保证两子域界面上的位移连续; Huerta 则采用直接耦合的方式, 在交界域内将相应的无网格法和有限元法的节点各自形成的逼近或插值函数迭加形成交界区域内任意点的逼近函数。程玉民、李九红则是将复变量无网格方法与有限元法耦合, 将求解域划分为复变量无网格子域及有限元子域, 各子域分别用复变量无网格法及有限元法建立基本方程, 在子域交界处设置虚拟有限元, 通过两子域交界面条件形成统一的求解方程。该法保留了无网格法可随意配点的优点; 交界面可不规则; 提高了无网格子域内的求解精度, 从而提高了问题的整体求解精度, 基本解决了 Belytschko 及 Huerta 耦合法存在的问题; 但在交界面附近区域误差较大^[277]。

8 结论

(1) 连续介质数值方法, 以有限元法为代表, 在理论、应用、软件开发等方面已臻成熟。重要的问题是扩大其应用领域, 继续完善软件功能, 特别是加强可视化及仿真技术的应用, 使其界面更加友好。

(2) 非连续介质数值方法, 在理论上尚不够成熟, 以损伤力学及断裂力学为代表的处理有缺陷模型的理论及方法, 仍需在大量实验基础上继续完善, 构成统一的破坏数值模型及理论框架;

表 2 无网格方法小结
Table 2 Brief summary of meshless methods

时 间	研究学者代表	名 称	近似函数	离散方案
1975 年	Perrone,Kao	广义有限差分法 (General Finite Difference Method for Arbitray Meshes)	核函数	配点法
1977 年	Lucyt	光滑质点流体动力学方法(SPH) (Smoothed Particle Hydrodynamics)		
1992 年	Nayroles	散射元法(DEM) (Diffuse Element Method)	移动最小二乘法	Galerkin 法
1994 年	Belytschko	无网格 Galerkin 法(EFGM) (The Element-Free Galerkin Method)	移动最小二乘法	Galerkin 法
1994 年	Amaratunga K	小波 Galerkin 法(WGM) (Wavelet-Galerkin Method)	小波级数	Galerkin 法
1995 年	Liu W K	再生核质点法(RKPM) (Reproducing Kernel Particle Method)	再生核近似	Galerkin 法
1995 年	Liu	多尺度再生核质点法(MSRKPM) (Multi Scale Reproducing Kernel Particle Method)	再生核近似	Galerkin 法
1995 年	Liu W K	小波质点法(WPM) (Wavelet Particle method)		
1995 年	Oden	Hp 云团(Clouds)法(HPCM) (Hp Clouds Method)	单位分解法	配点法
1996 年	Melenk J M	单位分解法(PUM) (Partition of Unity Method)	单位分解法	Galerkin 法
1996 年	Melenk J M	单位分解有限元法(PUFEM) (Partition of Unity Finite Element Method)	单位分解有限元法	
1996 年	Onate	有限点法(FPM) (The Finite Point Method)	移动最小二乘法	配点法
1996 年	Liu W K	移动最小二乘积分核法(MLSRKM) (Moving Least-Square Reproducing Kernel Method)	移动最小二乘积分核法	
1996 年	Liszka	Hp 无网格云团法(HPMCM) (Hp Meshless Clouds Method)	移动最小二乘法	配点法
1997 年	石根华	无单元流行法(MM) (Manifold Method)		
1998 年	Sukumar N	自然单元法(NEM) (Natural Element Method)		
1998 年	Oden	新型 Hp 有限元法(NCBHPFEM) (New Clo uds-Based Hp FEM)	有限元形函数	
1998 年	Arluri S N	无网格局部伽辽金法(MLPGM) (Meshless Local Petro-Galerkin Method)	移动最小二乘法	Petro-Galerkin 法
1998 年	Zhu T	局部边界积分法(MLBIEM) (Meshless Local Boundary Integral Equation Method)	移动最小二乘法	Petro-Galerkin 法
2000 年	Babuska	广义有限元法(GFEM) (Generalized Finite Element Method, GFEM)		
2000 年	田荣	有限覆盖无单元法(FCEFM) (Finite-Cover Eleme nt-Free Method)		
2000 年	张雄	紧支径向基函数无网格法 (Meshless Method Based on Collocation with Radial Basis Function)	紧支径向基函数	配点法
2001 年	Ohs R R	无网格配点法(PCM) (Meshless Point Collocation Method)	再生核近似	配点法
2001 年	张见明	杂交边界点法(HBPM) (Hybrid Boundry Point Method)	移动最小二乘法	Galerkin 法
2001 年	张雄	最小二乘配点无网格法(LSCMM) (Least-Square Collocation Meshless Method)	移动最小二乘法	最小二乘配点法
2001 年	张雄	加权最小二乘无网格法(WLSMM) (Weight Least-Square Meshless Method)	移动最小二乘法	加权最小二乘法
2004 年	程玉民, 李九红	复变量无网格方法 (Meshless Method with Complex Variables)	复变量移动最小二乘法	Galerkin 法

(3) 流形元法及无网格方法, 为建立连续——非连续统一数学模型奠定了基础, 但仅有二维问题的初步成果, 没有可供推广的成熟的软件, 实际工程的应用还处在探索阶段, “任重道远”, 要达到有限元、边界元等数值方法的应用水平, 还相差甚远。

(4) 如何在丰富众多高效的 FEM 商业软件中, 引入处理非连续问题的功能, 作到连续与非连续的统一, 是当前可变形固体力学应着重考虑的问题。从这一角度出发, 数值流形元法、无单元法与 FEM 耦合问题的研究及相应的软件开发, 更具有开拓发展的价值。

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